



UNIVERSIDAD NACIONAL DE COLOMBIA

SEDE BOGOTÁ

FACULTAD DE INGENIERÍA

ÁREA CURRICULAR DE INGENIERÍA CIVIL Y AGRÍCOLA

DOCTORADO EN INGENIERÍA - INGENIERÍA CIVIL



Doctorado en Ingeniería

Ingeniería Civil

Reliability Assessment of Rock Slopes by Discontinuous Deformation Analysis

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Content

1. Introduction
2. Rock Mass modeling
 1. Discrete models
3. Reliability Assessment
 1. Random set theory
4. Problem definition
5. Methodology Outline

Introduction



💧 Rock mass features

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(1)



(2)

Rock mass features



(3)



(4)

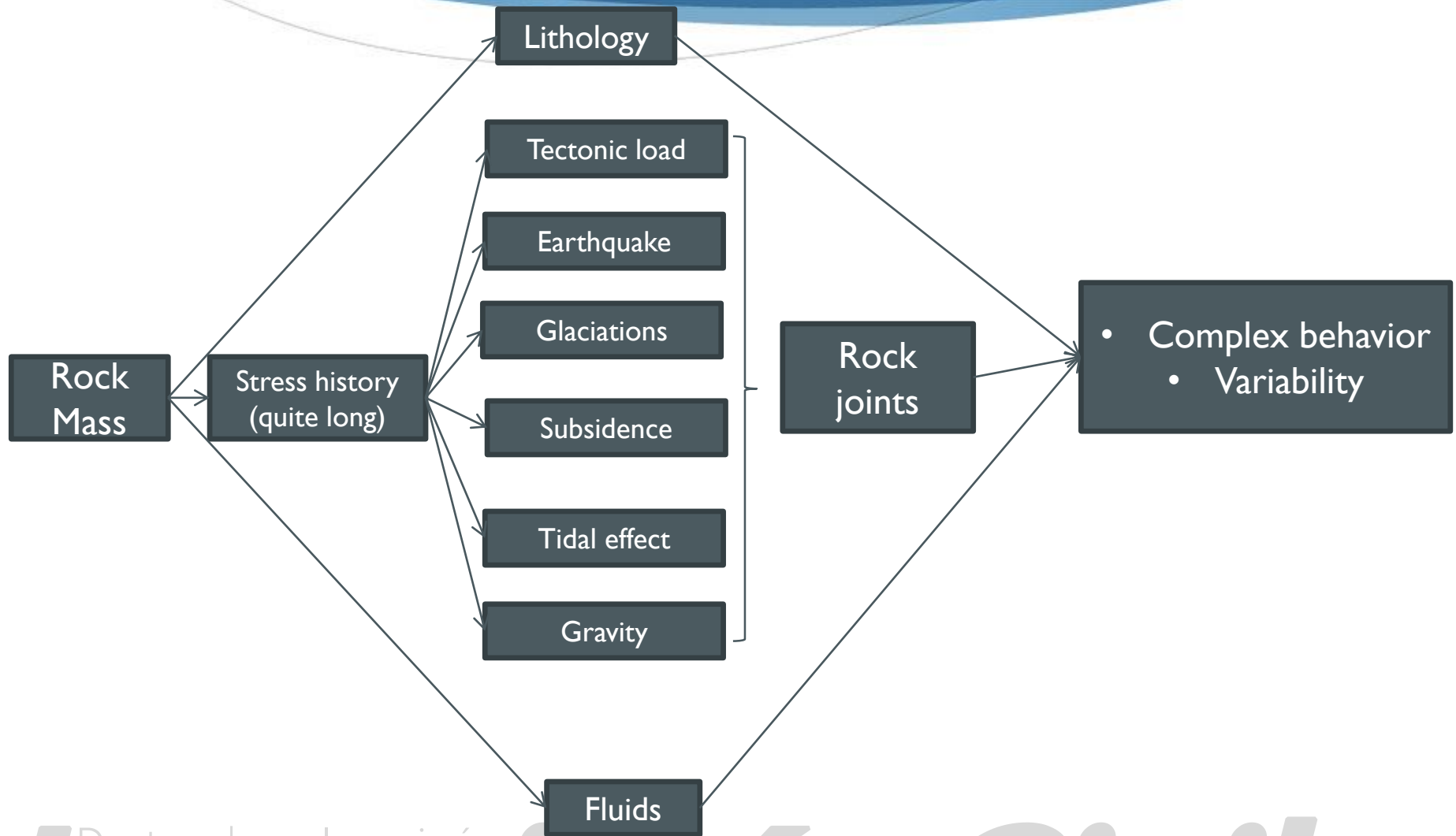
Rock slope failure

1. Vertical joint set in a limestone mine. Duitama Boyacá. Dip direction slightly changes from one joint to the next one
2. Rock slope in a limestone mine. Duitama, Boyacá.
3. Phosphate mine. Rock slope failure due to underground tunnel excavation. Firavitova, Boyacá
4. Rock slope failure in a limestone mine, controlled by the vertical joint set. Limestone mine.

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Introduction

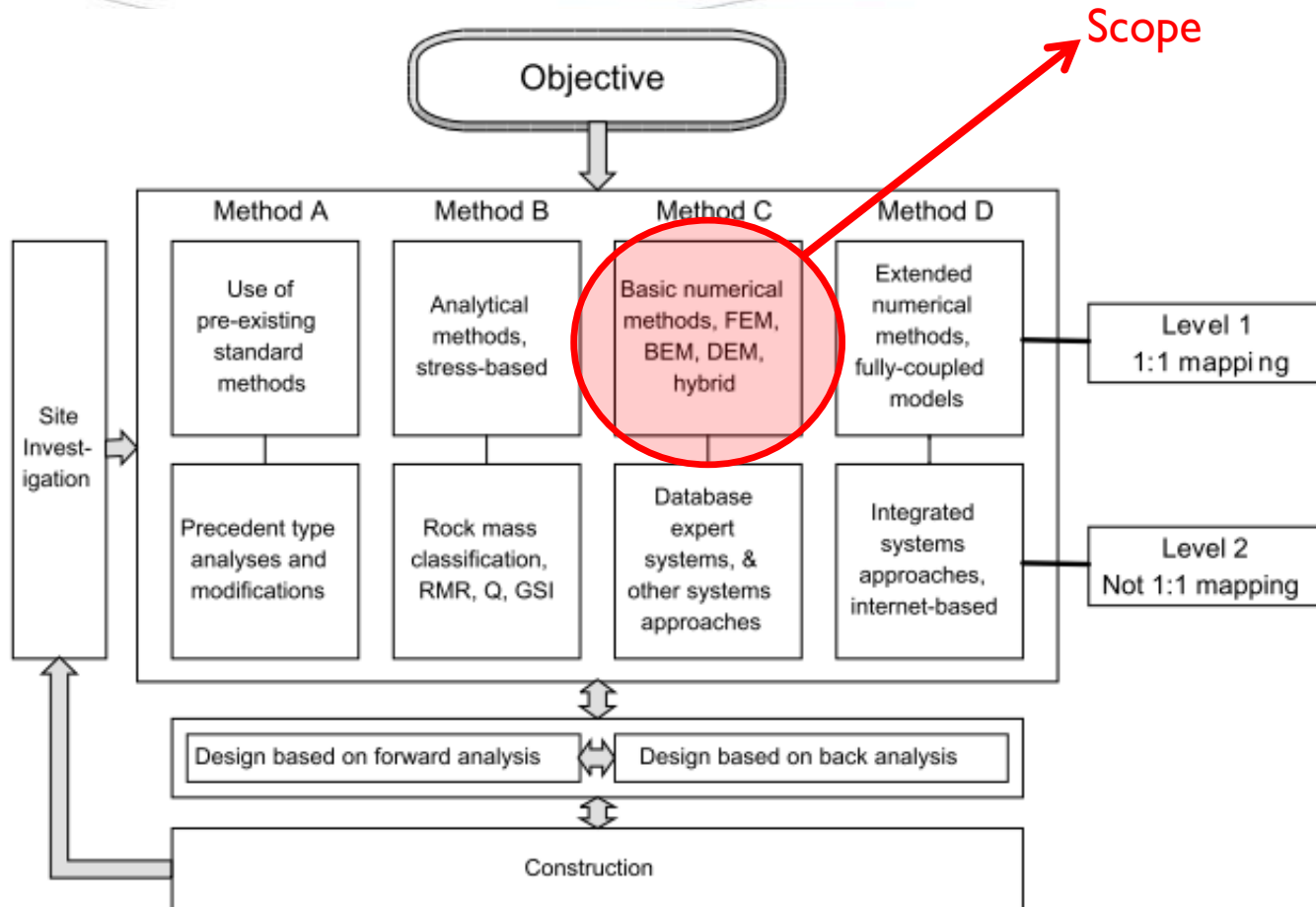


Rock Mass Modeling

Model should have the following features (Jing, 2007):

- ◆ Represent the model rock mass geometry, including fractures and boundaries
- ◆ Represent the initial conditions of the problem
- ◆ Include the initial state of stresses
- ◆ Consider appropriate constitutive laws for both rock block and rock joints
- ◆ Include scale and time effects
- ◆ When environmental impact is an issue, model should consider a physical and chemical coupling

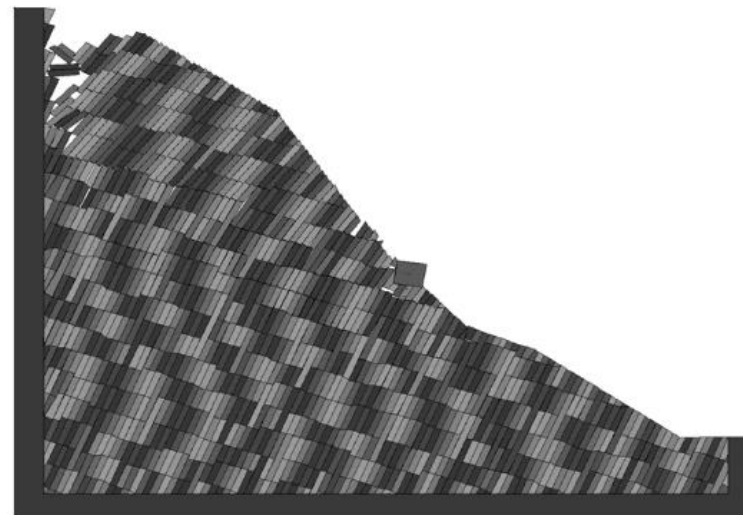
Rock models



After Hudson 2001

Numerical methods

- These can be either continuum or discontinuum (Jing and Hudson, 2002; Jing 2003)
- Discrete Elements methods
- Main features (Cundall, 1992)
 - Allow finite displacement and rotation of discrete bodies, including detachment
 - Automatically recognize new contacts between bodies during calculations



Discontinuum based methods

◆ Methods

- ◆ Distinct Elements Method (Cundall, 1971)
- ◆ Discontinuous Deformation Analysis (DDA) (Shi, 1985)
- ◆ Numerical Manifold Method (NMM) (Shi, 1993)

◆ Three main issues (Jing, 2007):

1. Representations of contacts
2. Detection and revision of contacts during execution
3. Representation of solid materials

Discontinuous Deformation Analysis

Problems to solve (Jing, 2007)

- Representation of the fracture systems according to field mapping
- Identification of block-fracture system with fracture system regularization
- Formulation of motion equation (equilibrium equation)
- Representation of rock and fracture constitutive laws
- Representation of contact detection and evolution
- Integration of equations of motion and fluid flow
- Proper data structure for efficient computer programs

Uncertainty

Classified into:

- ◆ *Aleatory (inherent) uncertainty*, representing the natural randomness of a variable. Cannot be reduced
- ◆ *Epistemic uncertainty*, representing the uncertainty due to lack of knowledge on a variable.
 - ◆ Measurement uncertainty
 - ◆ Statistical uncertainty (due to limited information)
 - ◆ Model uncertainty

Can be reduced by:

- ◆ Increasing the number of tests
- ◆ Improving measurement methods

Sources of uncertainty in rock slopes:

- ◆ Discontinuity geometry (orientation, size, spacing)
- ◆ Discontinuity resistance either directly described by friction angle and cohesion or indirectly through roughness/irregularities and infillings
- ◆ Resistance of intact rock related to the persistence effect
- ◆ Water pressure/Ice pressure
- ◆ Vegetation, support
- ◆ Slope geometry

Reliability assessment

Technique to deal with uncertainty

Limit state function $g(x)$:

$$g(x) = \begin{cases} > 0 & \text{for the safe state of the structure} \\ < 0 & \text{for the failure of the structure} \end{cases}$$

$\mathbf{X} = (X_1, X_2, \dots, X_n)$ Random variables

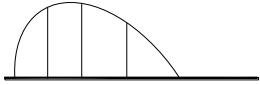
$$g(x) = \alpha^* - \alpha(X_C, X_E, X_F)$$

$$g(x) = \left(\frac{\delta}{L}\right)^* - \left(\frac{\delta}{L}\right)^C$$

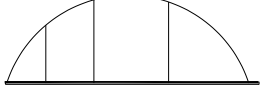
- ◆ FOSM Reliability Index. (First-Order, Second-Moment) approach (Ang & Tang 1984)
- ◆ *First- and second-order reliability methods (FORM and SORM)* Hasofer & Lind (1974)
- ◆ *Monte Carlo Simulation (improved by Latin Hypercube)*
- ◆ *RST Random Sets Theory*

PROBABILITY DENSITY FUNCTION

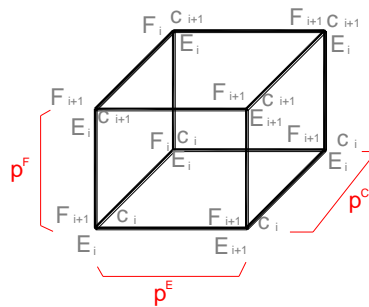
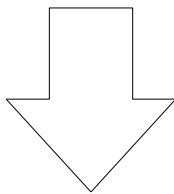
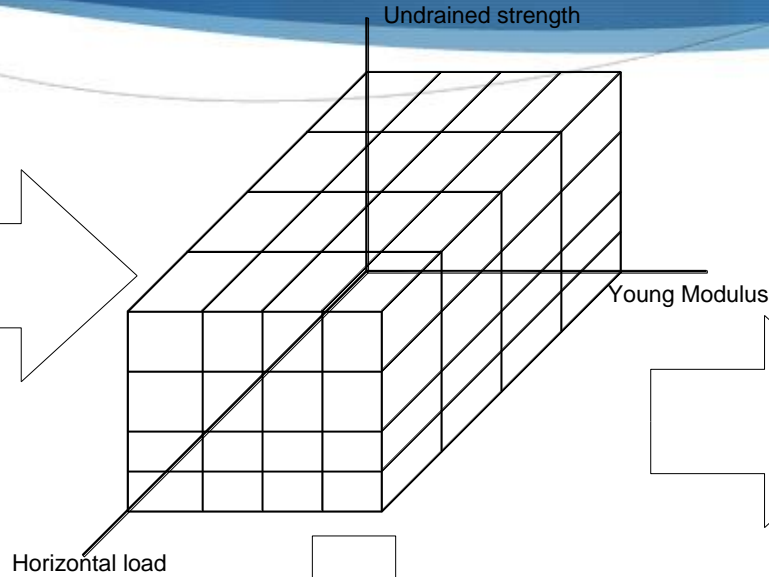
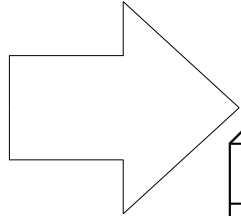
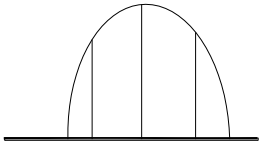
Undrained strength



Young Modulus



Horizontal load



FEM MODEL

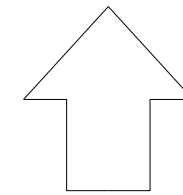
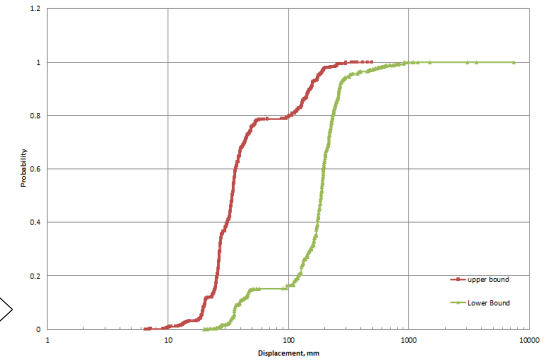
$$d = g^{\text{high}}(c_i, E_i, F_{i+1})$$

$$d = g^{\text{low}}(c_{i+1}, E_{i+1}, F_i)$$



JOINT PROBABILITY

$$p^j = p^c * p^e * p^f$$



FEM MODEL

$$d = g^{\text{high}}(c_i, E_i, F_{i+1})$$

$$d = g^{\text{low}}(c_{i+1}, E_{i+1}, F_i)$$

$$D_j = (d^{\text{low}} \ d^{\text{high}})$$

With a basic probability assignment

$$p^j$$

Assume that probability mass of each interval is concentrated at the lower bound of each focal element

Sort lower bound from the smallest to the greatest

Define interval from sorted data

Assign probability concentrated on the low bound of the original focal element to each new interval

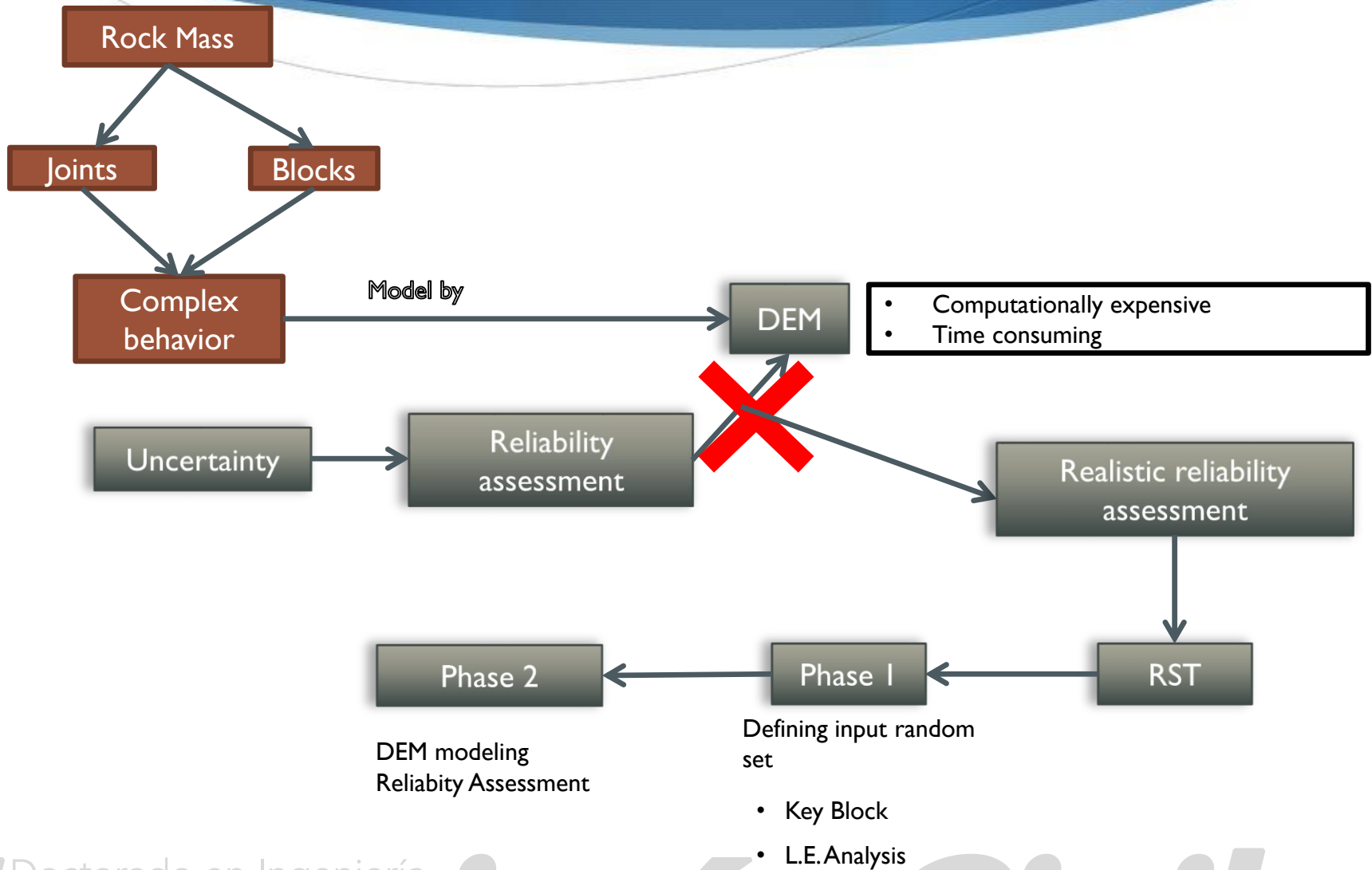
Sum up the probability assigned to each interval

Probability function computation (upper)

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Problem definition



Methodology outline

Field work

- Rock mass data collection
- Rock block sampling

Model generation

- Block models generation

Input random Sets

- Key Block Analysis
- LE Analysis

Discrete modelling

- Discrete element modeling of all possible random input combinations

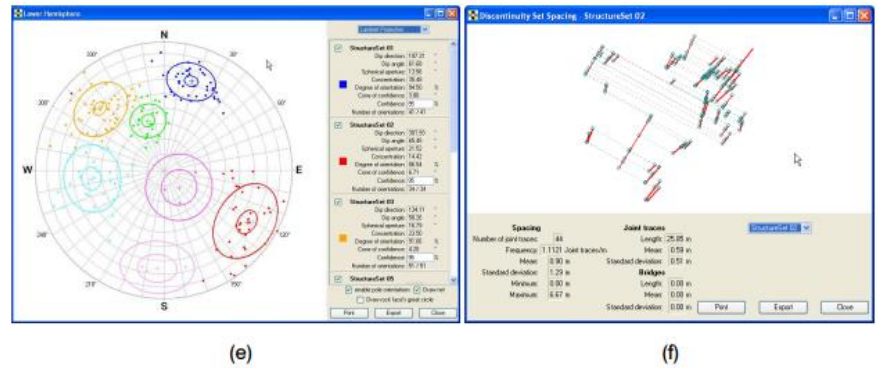
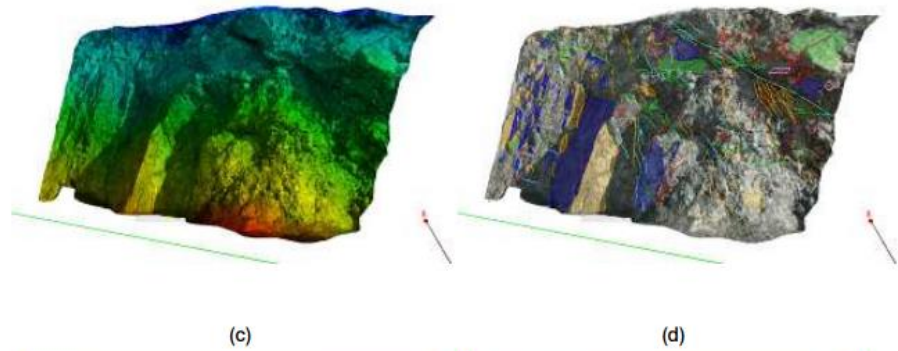
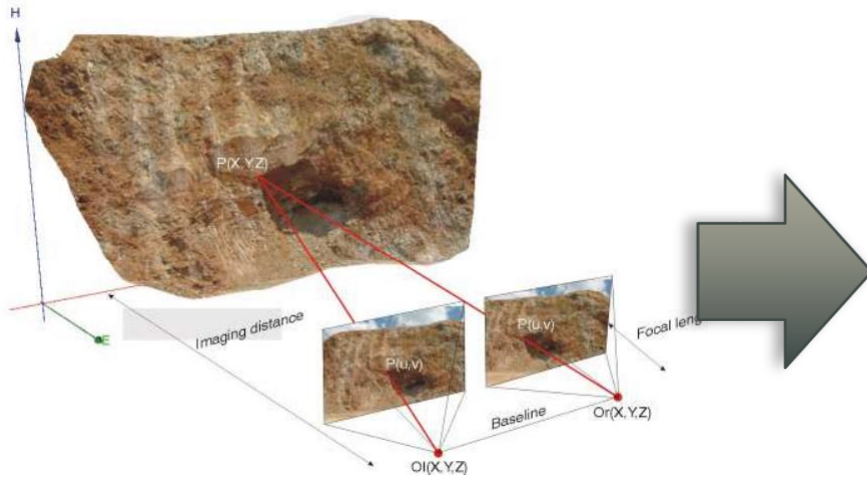
Reliability Assessment

- Performace function definition
- Cumulative probability curves computation (displacements on slope face)
- Probability of failure calculation

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Field Work



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THANKS

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